

## **Development of G13.1**

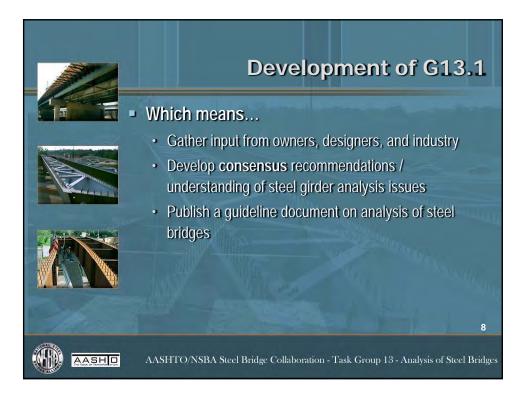


AASHO

"Develop and maintain consensus recommendations on the applicability, advantages, and disadvantages of analysis techniques for various types of steel girder bridges"

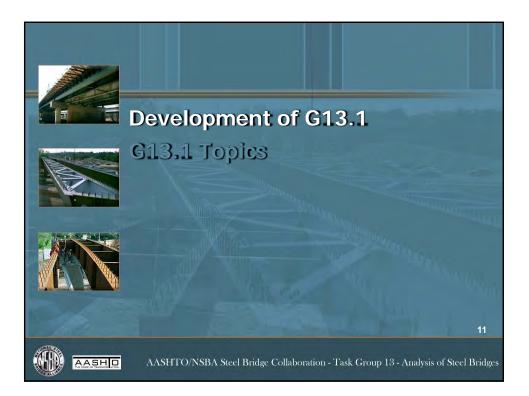
**Mission Statement** 

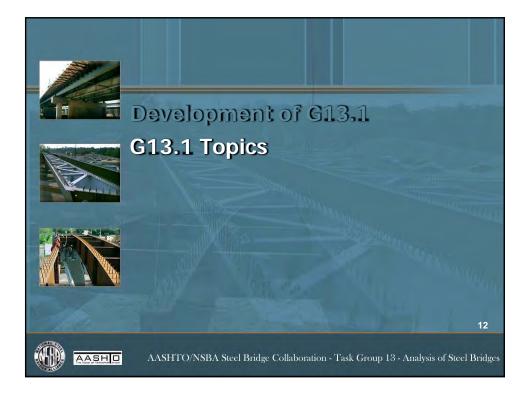
AASHTO/NSBA Steel Bridge Collaboration - Task Group 13 - Analysis of Steel Bridges

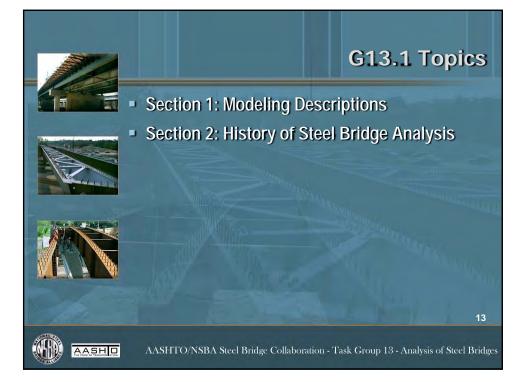


	Developr Timeline of TG13's efforts	nent of G1	3.1
	Activity	Date	
Researces!	Initial Meeting of TG 13	May 9, 2007	
	Survey of Current Practice	Summer 2007	
	Prepared Outline of G13.1	Winter 2007/2008	-
	Assigned Authors for G13.1	June 2008	Lines of Lot
	Reviewed Draft of G13.1	November 2009	
	Collaboration Ballot of G13.1	April 2010	
	Initial AASHTO T-14 Review of G13.1	August 2010	No/22
	AASHTO SCOBS Approval of G13.1	May 2011	1.000
Nº 10	Publication of G13.1	August 2011	
All and	From initial meeting to publication: > 4	lyears	9
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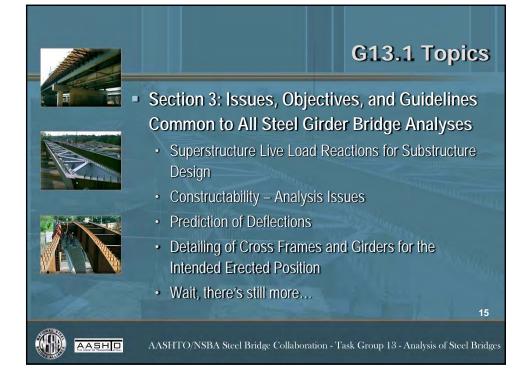
Development of G13.1     Formal development of consensus			1
	Review	Comments Addressed	
	October 2009 Review by TG13	411	
	March 2010 Review by TG13	193	
	April 2010 Review by Industry	35	
	October 2010 Review by T-14	5	
	February 2011 Review by T-14	18	
	May 2011 Review by AASHTO Editor	95	
	Total Comments Addressed	757	ui)
Cher Ca		1	10
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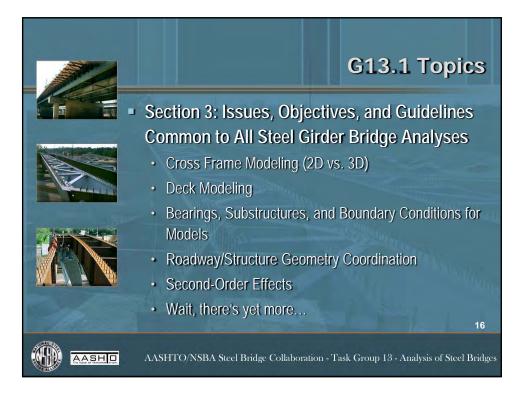


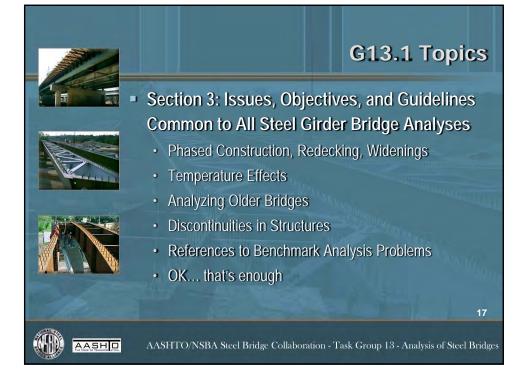




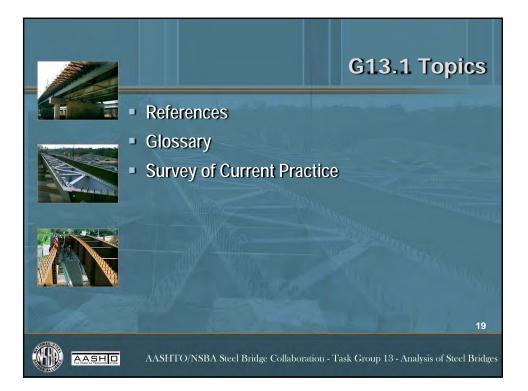
	G13.1 Topics
	Section 3: Issues, Objectives, and Guidelines
	Common to All Steel Girder Bridge Analyses
	Behavior Considerations
	Section Property Modeling Considerations
	Loads on the Permanent Structure
	Strength Design
	<ul> <li>Inelastic Design</li> </ul>
	<ul> <li>Fatigue Analysis and Evaluation</li> </ul>
1 des	Wait, there's more     14
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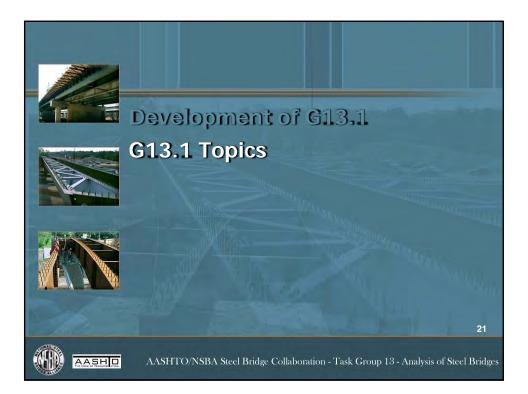


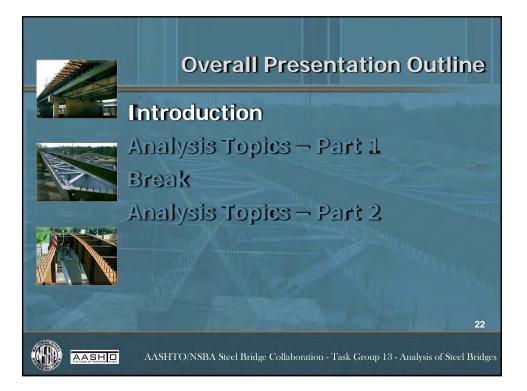


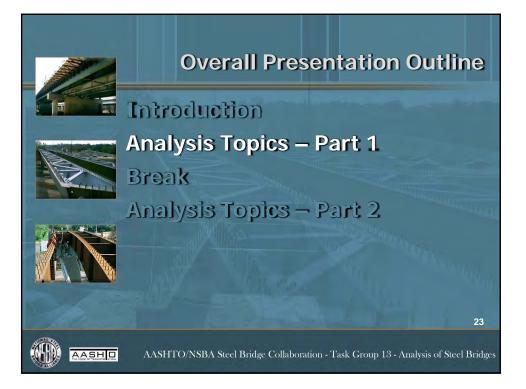
	G13.1 Topics
	Section 4: Analysis Guidelines for Specific
(8.1 - Contractor of the second	Types of Steel Girder Bridges
	Plate Girders – General Issues
	<ul> <li>Tangent Steel Plate Girders and Rolled Beams</li> </ul>
	Curved Steel Plate Girders and Rolled Beams
	<ul> <li>Tub Girders – General Issues</li> </ul>
	Tangent Steel Tub Girders
	Curved Steel Tub Girders
1 december of	<ul> <li>Bridges with Significantly Complex Framing         <sup>18</sup></li> </ul>
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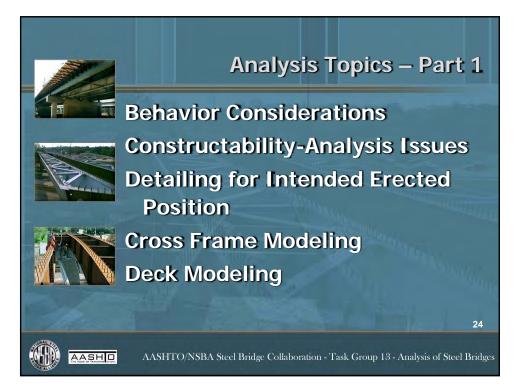








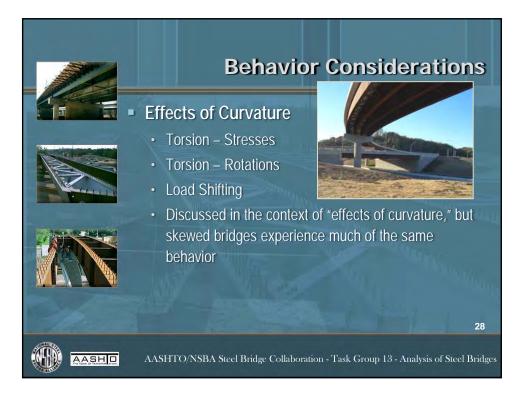


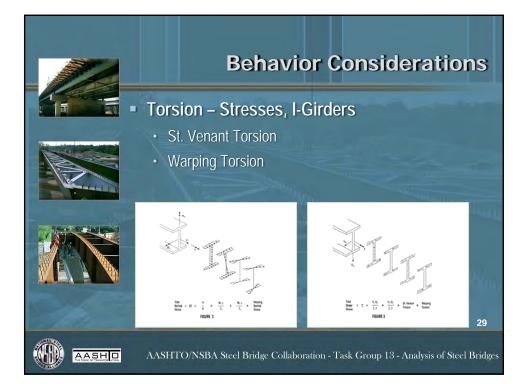


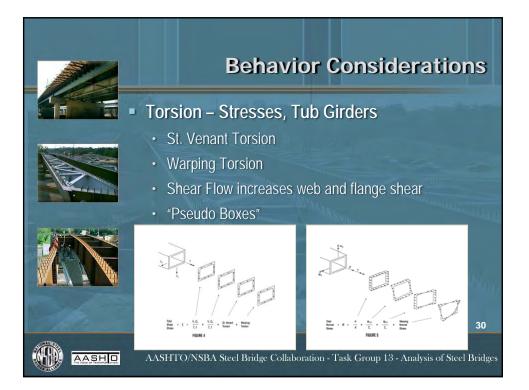


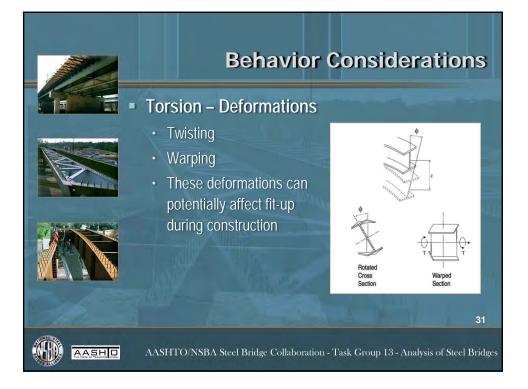


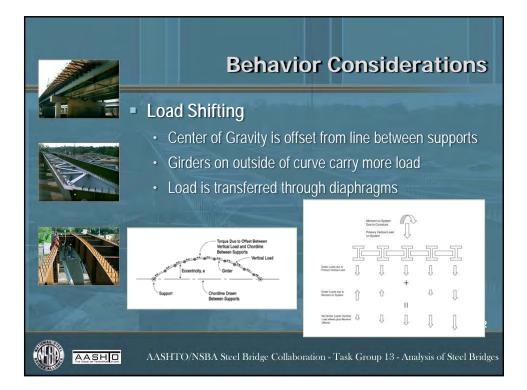


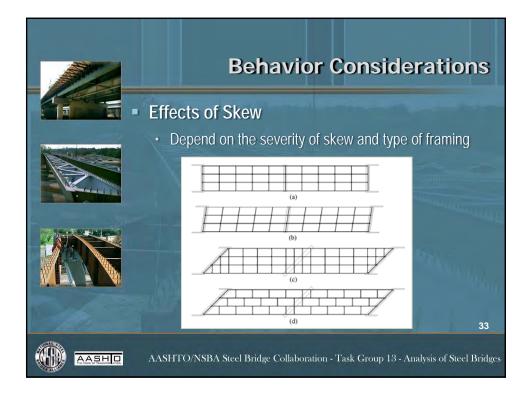


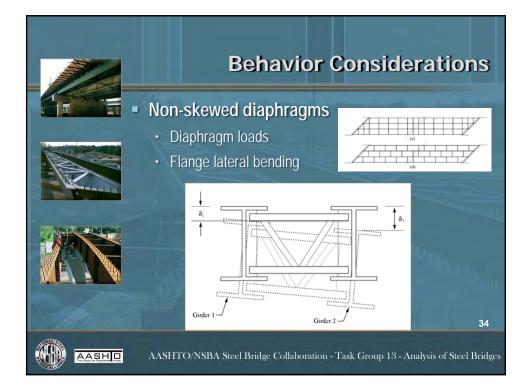


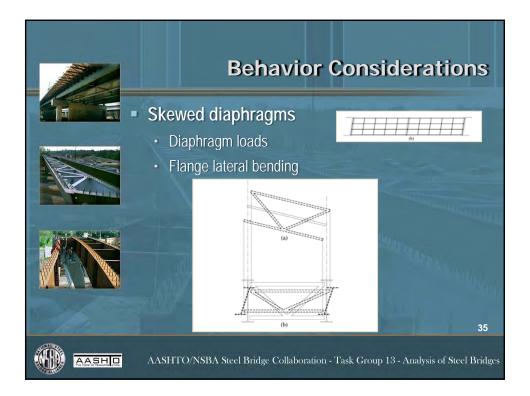




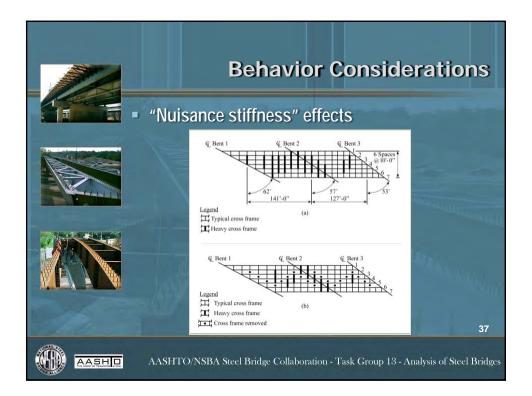


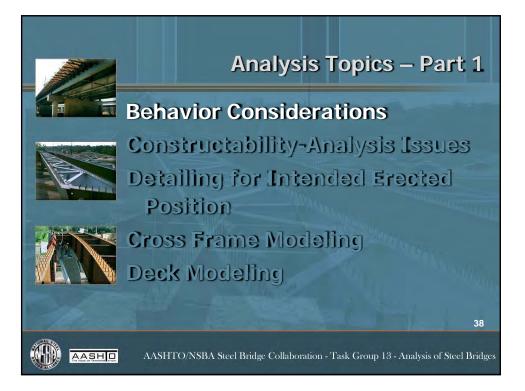


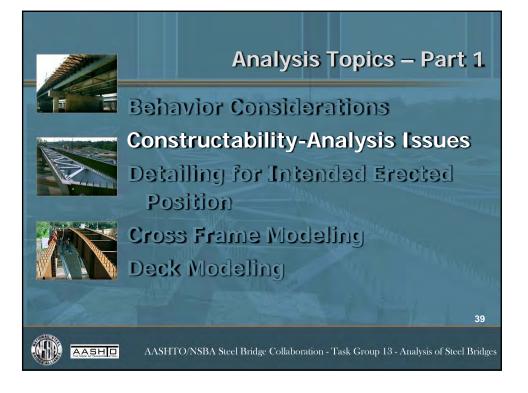


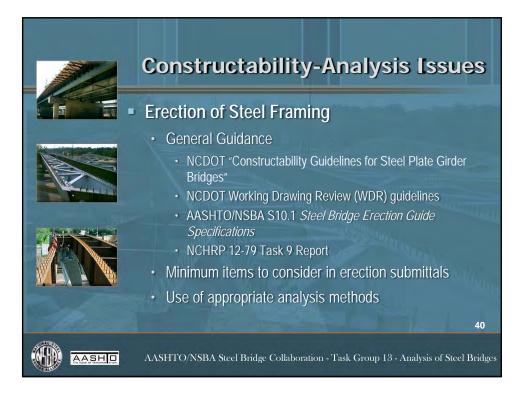


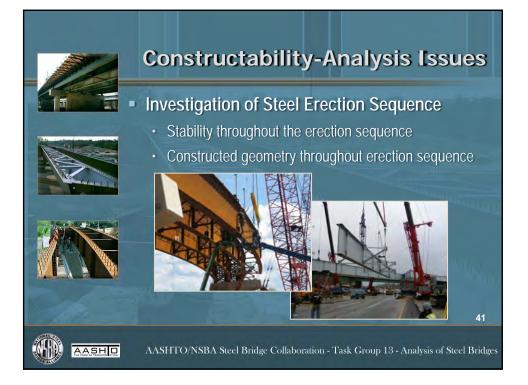


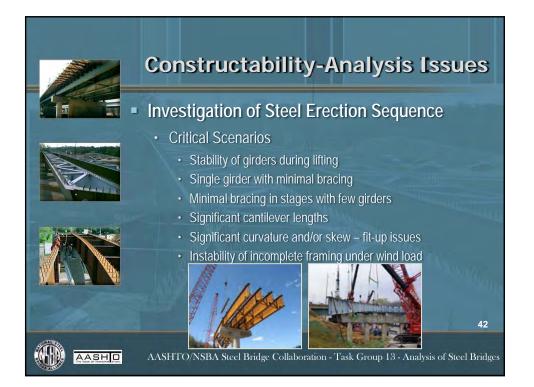


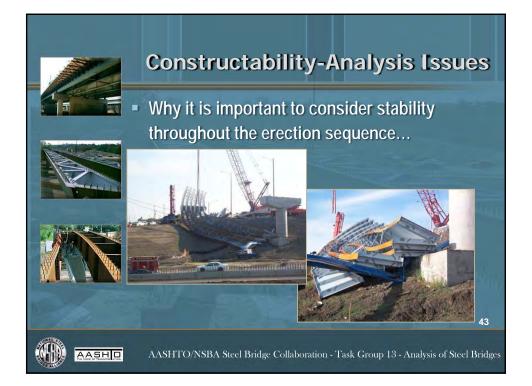


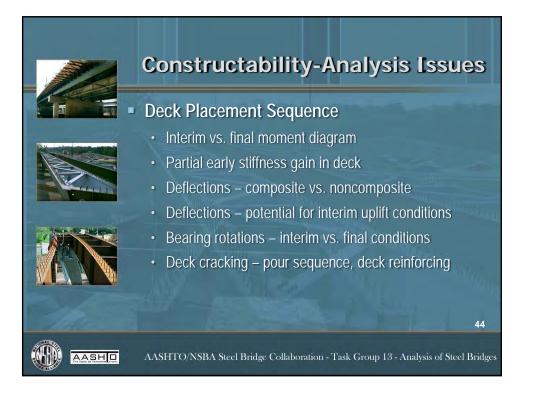


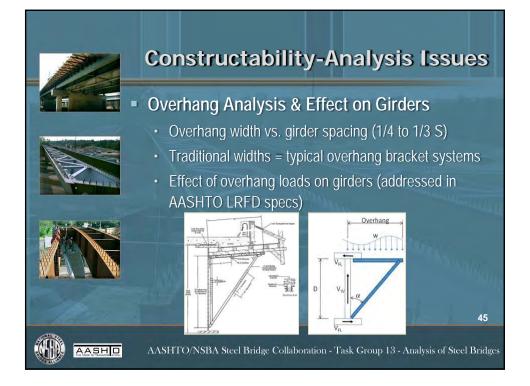


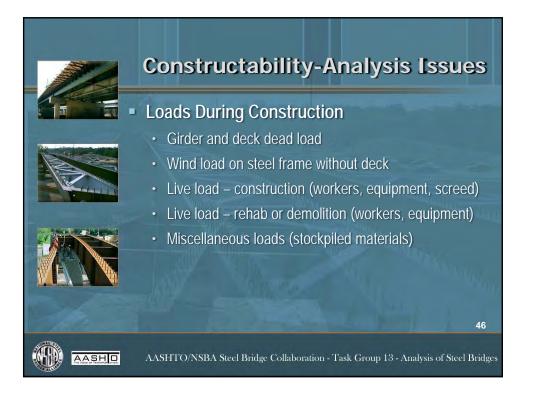


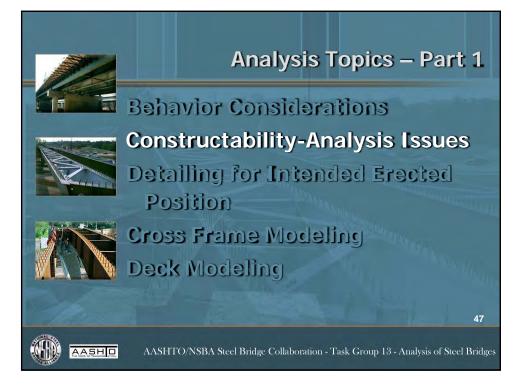


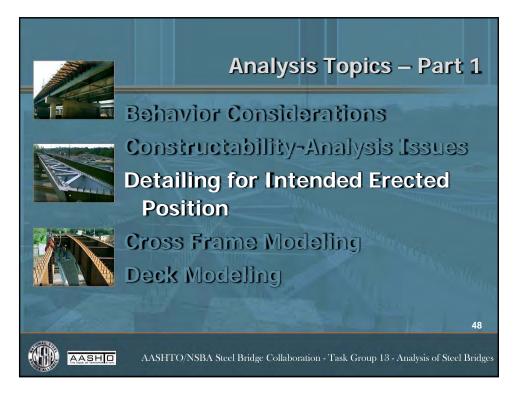


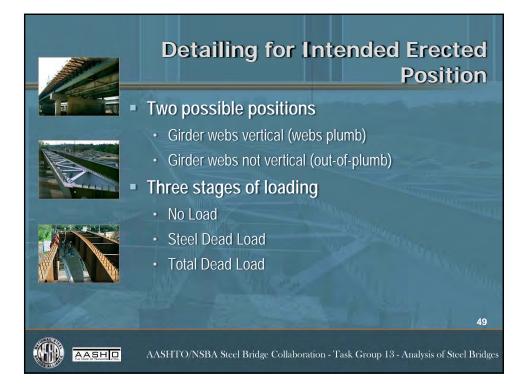


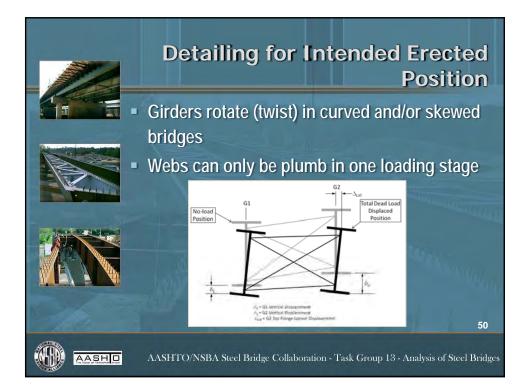


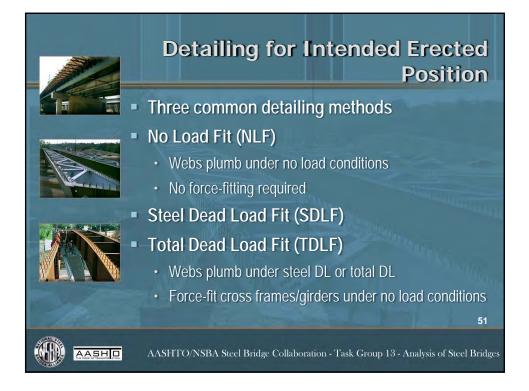


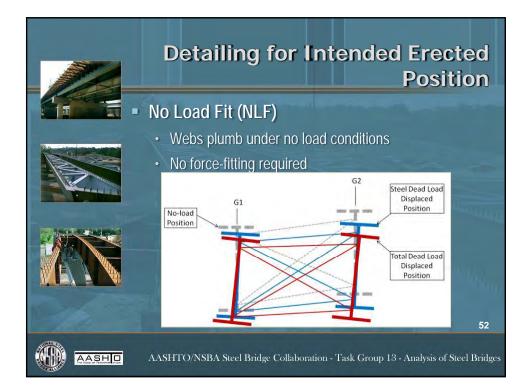


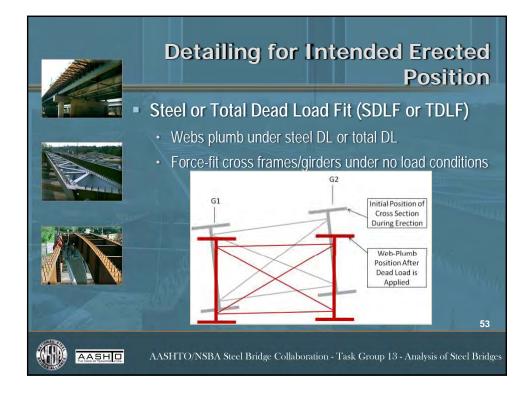




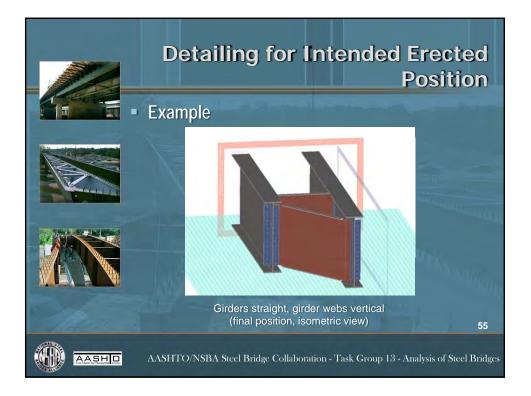


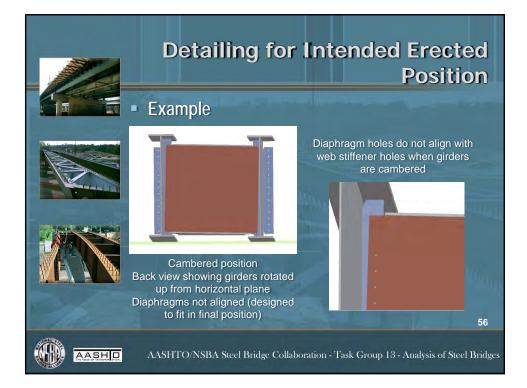


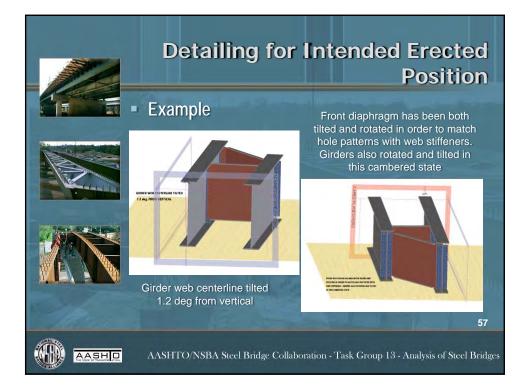


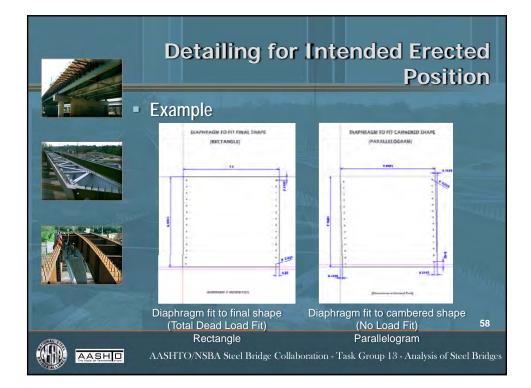


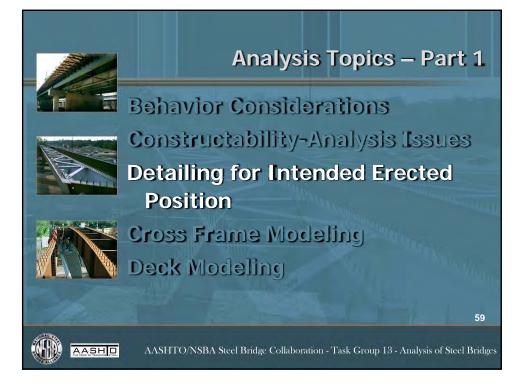
Detailing for Intended Erected Position
Considerations
<ul> <li>Detailing <ul> <li>Cross frame geometry is different for NLF vs. SDLF vs. TDLF</li> </ul> </li> <li>Fit-up <ul> <li>Plans must clearly specify detailing method or there may be fit-up issues during construction</li> </ul> </li> <li>Forces <ul> <li>NLF = Forces in final condition from traditional analysis</li> <li>SDFL, TDLF = Technically a "lack of fit" analysis is required</li> </ul> </li> <li>See NCHRP 12-79 Task 8 Report for more guidance</li> </ul>
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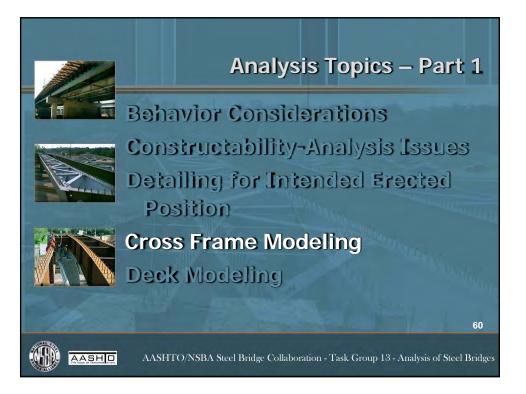


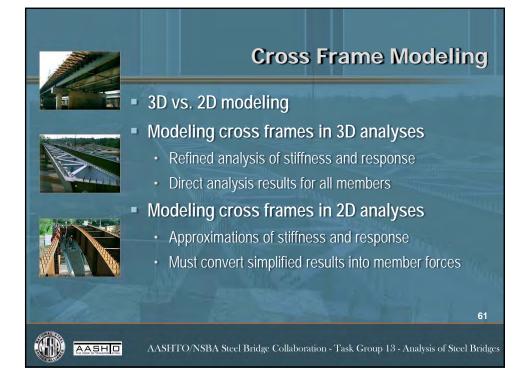


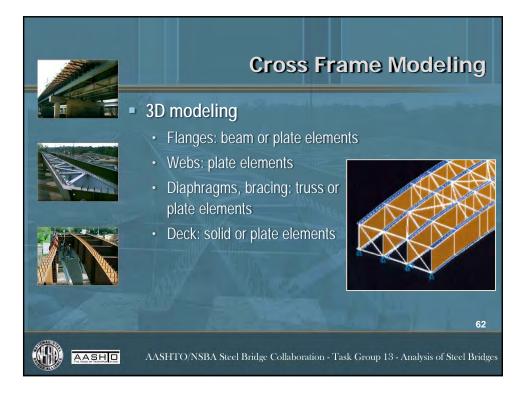


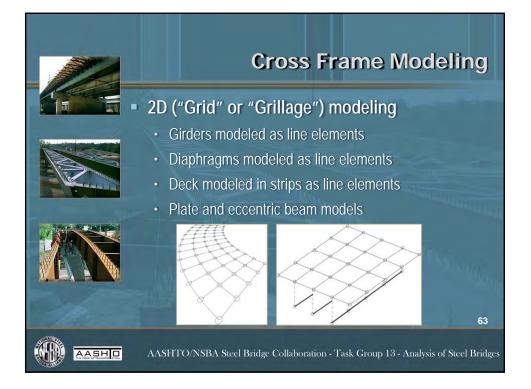


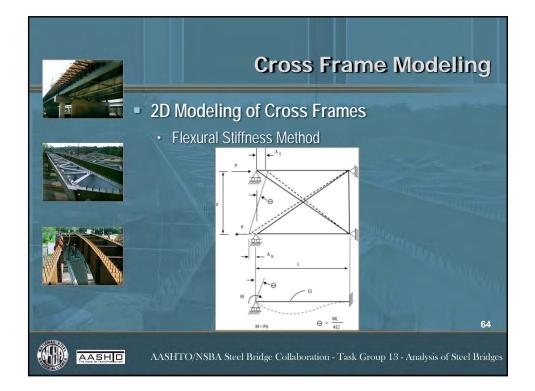


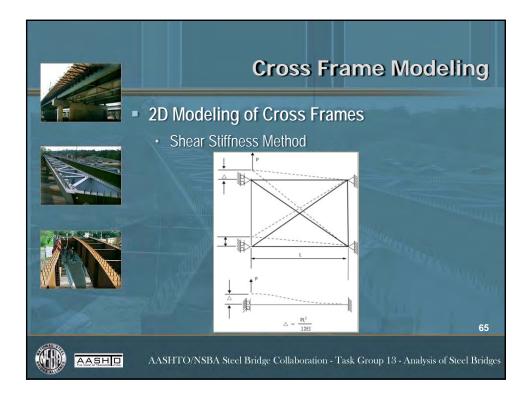


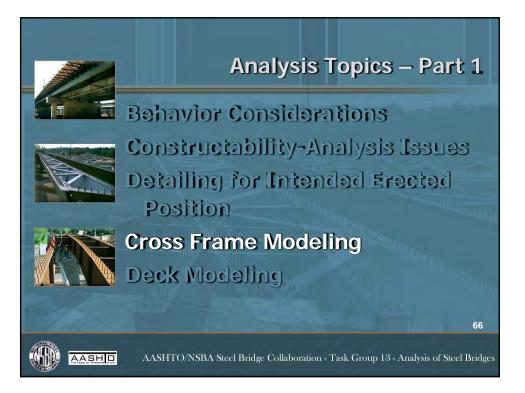


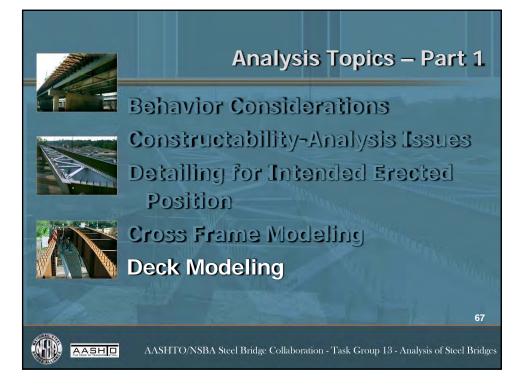


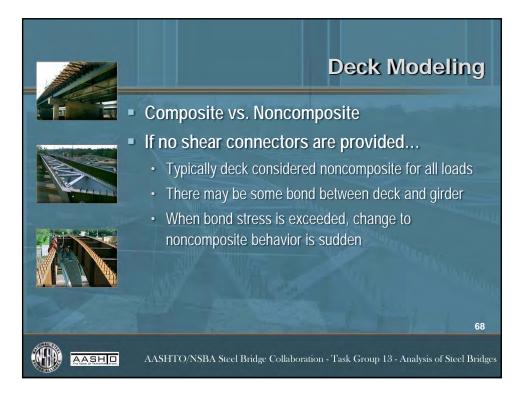


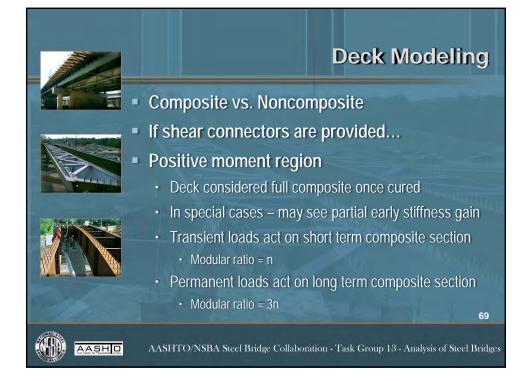




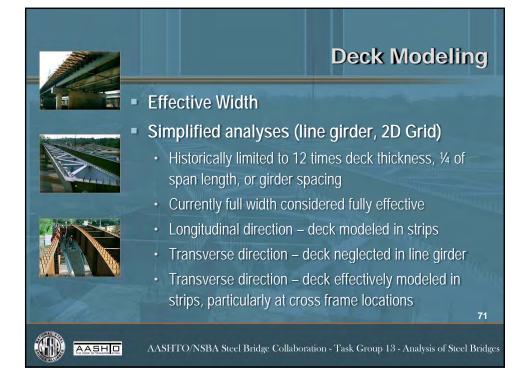


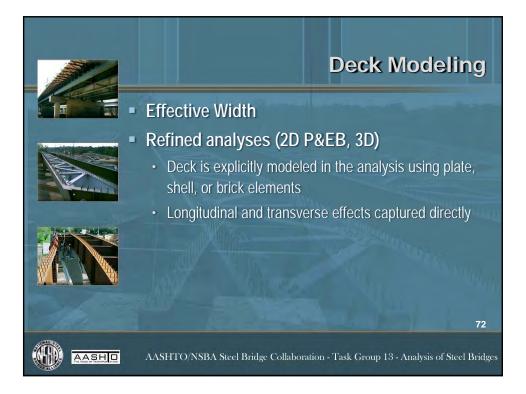


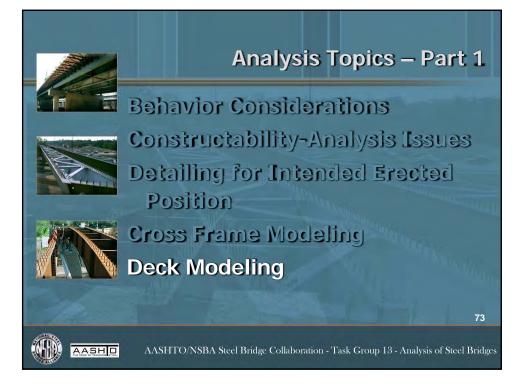


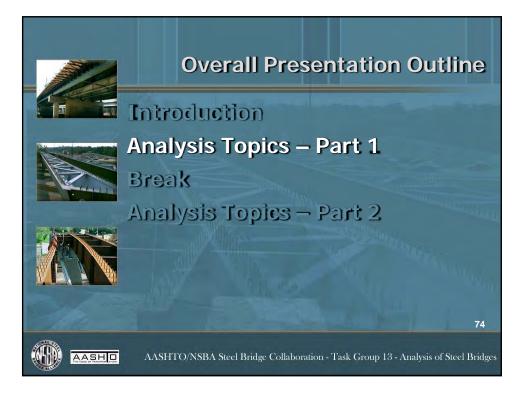


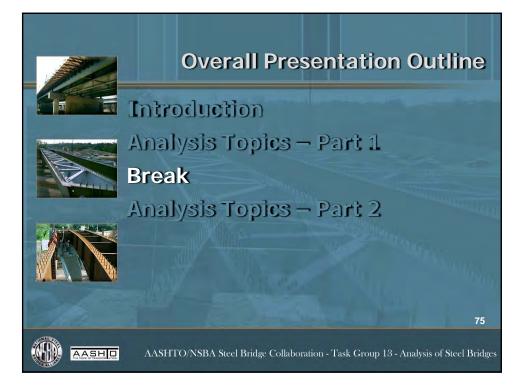
	Deck Modeling
	Composite vs. Noncomposite
-	If shear connectors are provided
	Negative moment region
	Without shear connectors in negative moment region
	No composite action
	<ul> <li>Deck concrete and longitudinal reinforcing considered totally ineffective</li> </ul>
	<ul> <li>With shear connectors in negative moment region</li> </ul>
	<ul> <li>Deck longitudinal reinforcing acts as part of composite section</li> </ul>
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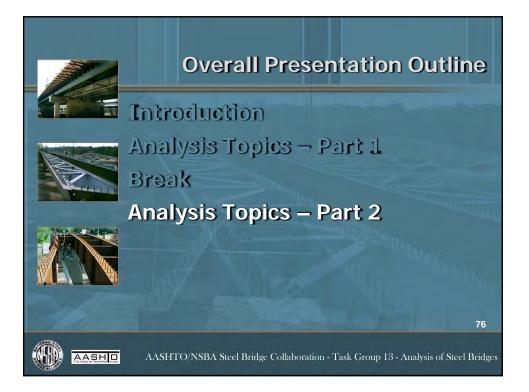




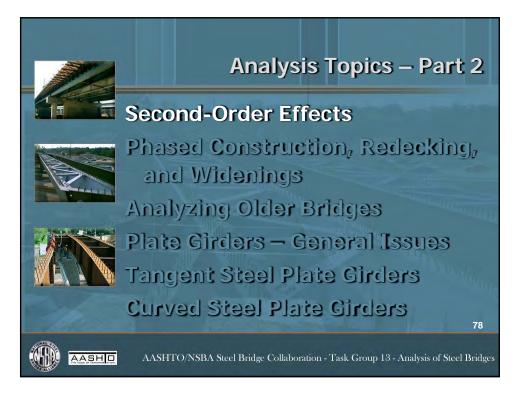


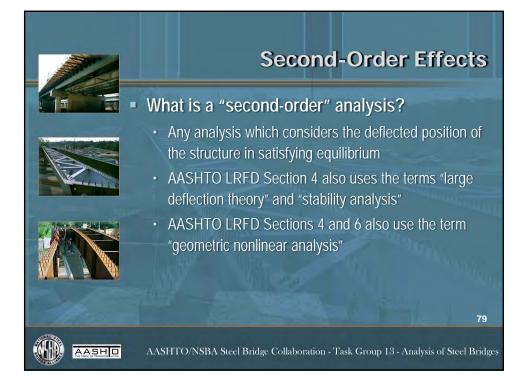




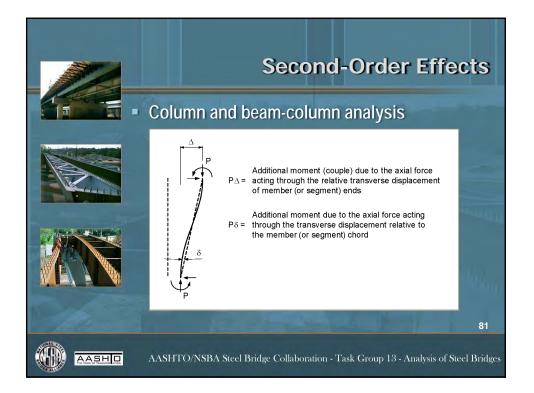




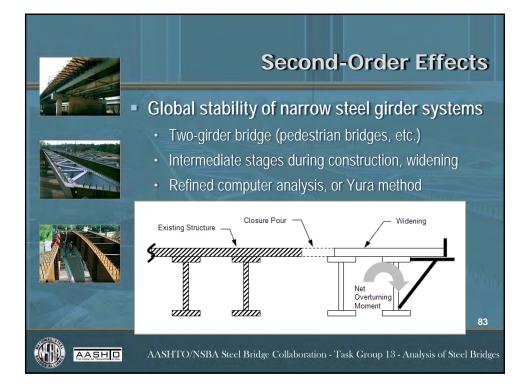


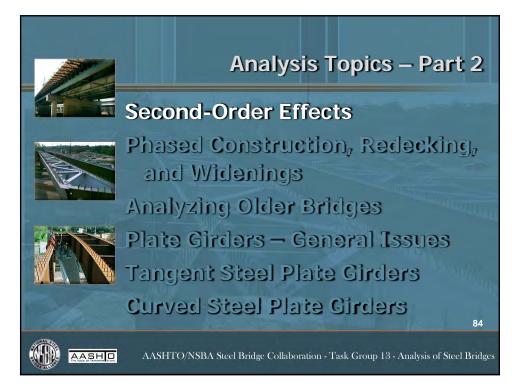


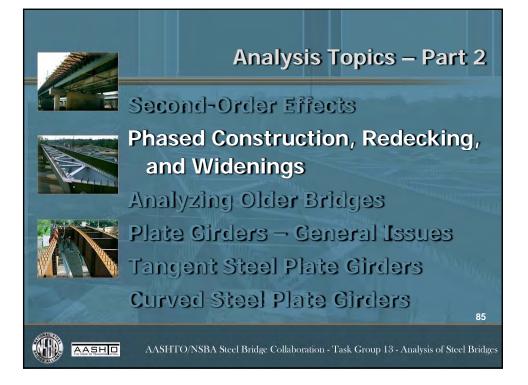
<b>Second-Order Effects</b> Generally two types of second-order analyses
<ul> <li>Buckling analysis</li> <li>Eigenvalues (buckling load levels)</li> <li>Eigenvectors (buckling modes)</li> <li>"K-factors" are derived from eigenvalue buckling analyses</li> </ul>
P-delta analysis (P-Δ, P-δ)         • Effects of load times relative displacement are considered incrementally via "iterative analysis"         80         AASHTO/NSBA Steel Bridge Collaboration - Task Group 13 - Analysis of Steel Bridges

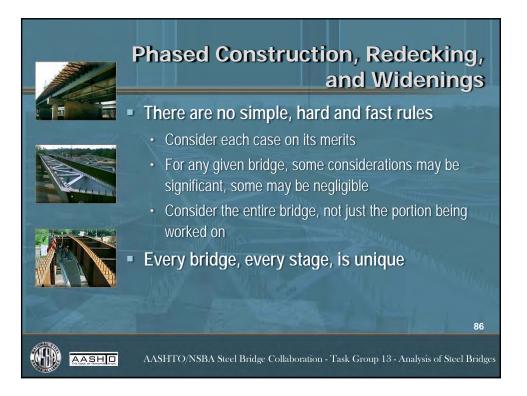


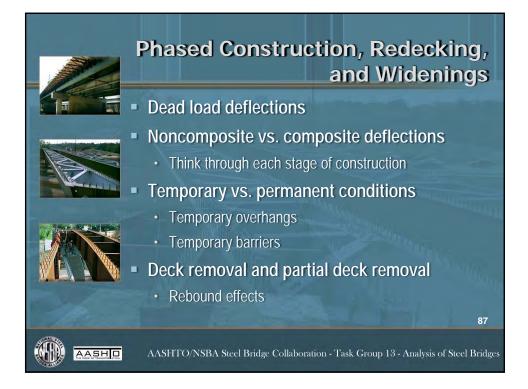


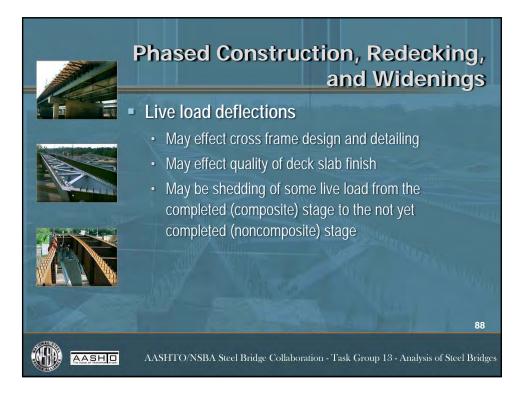


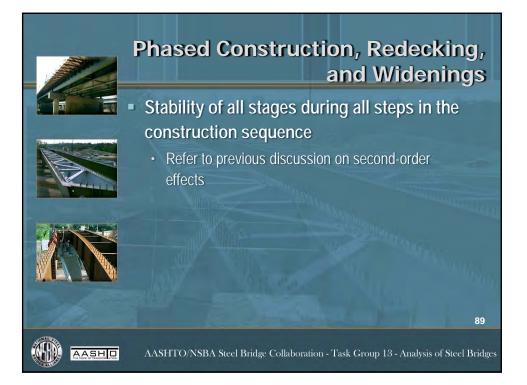


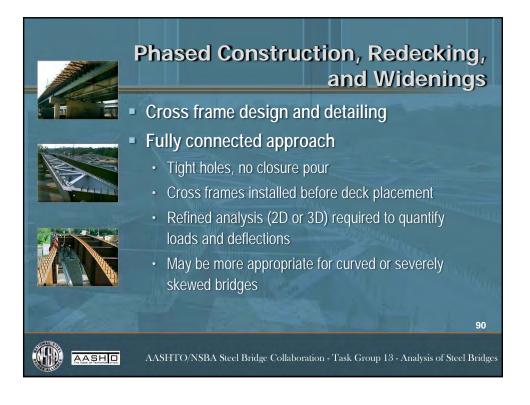


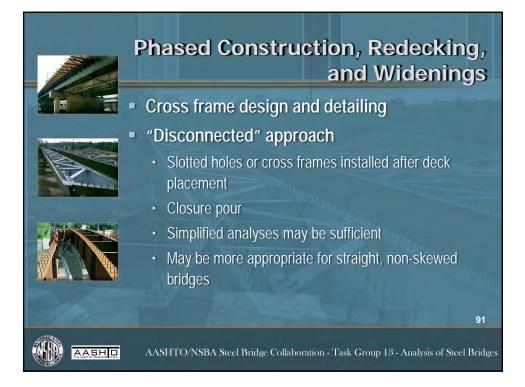


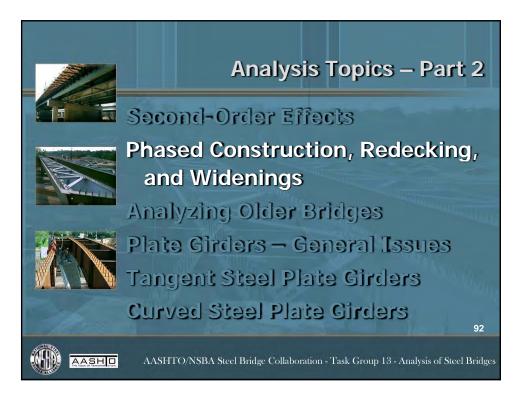


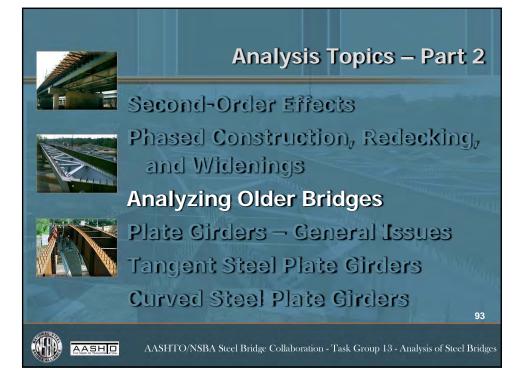


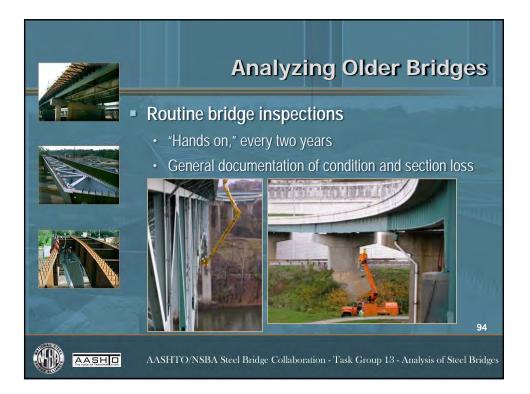


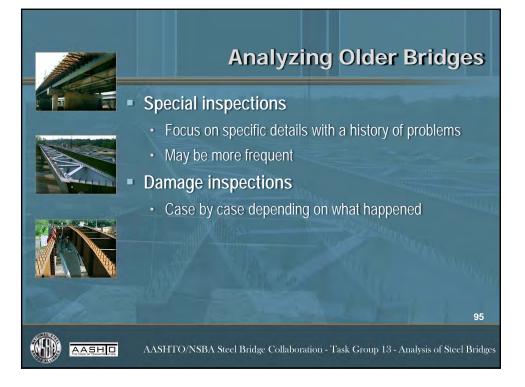


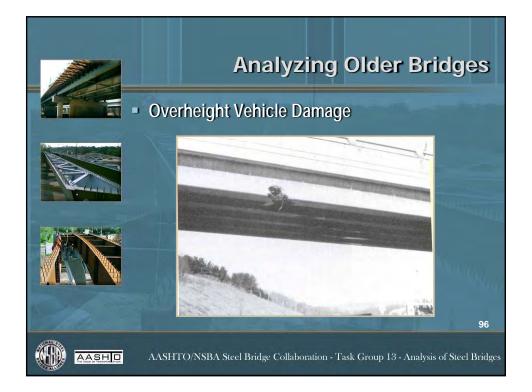


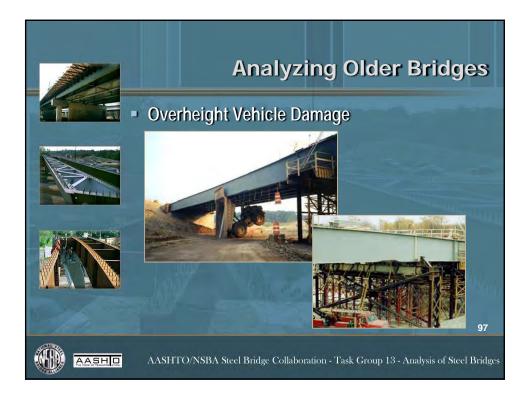


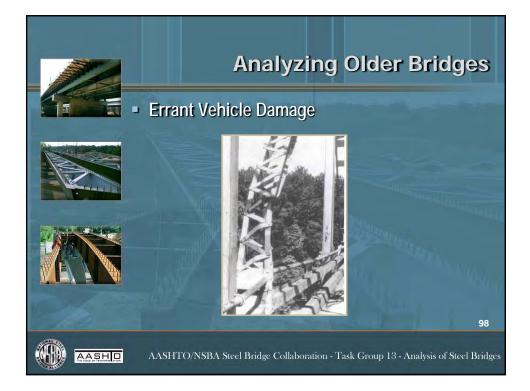


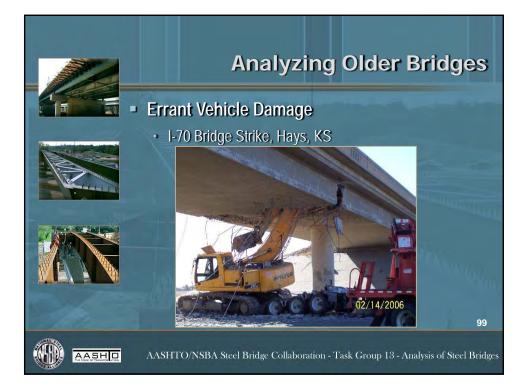


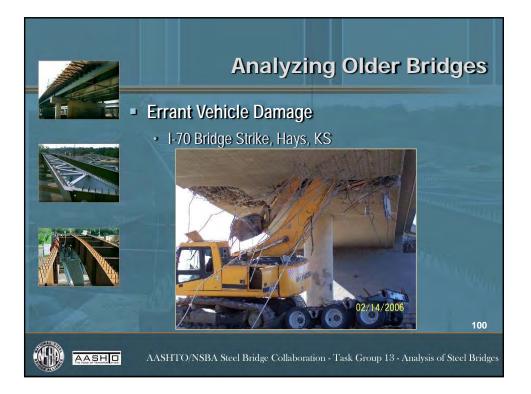


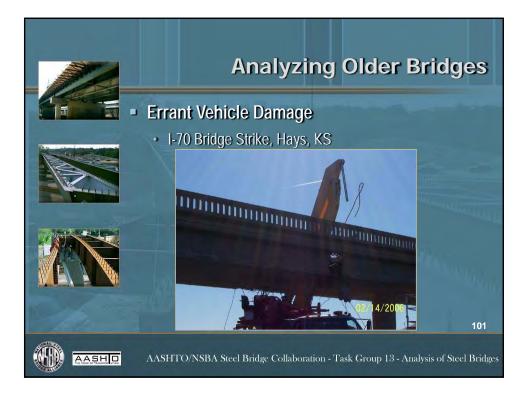


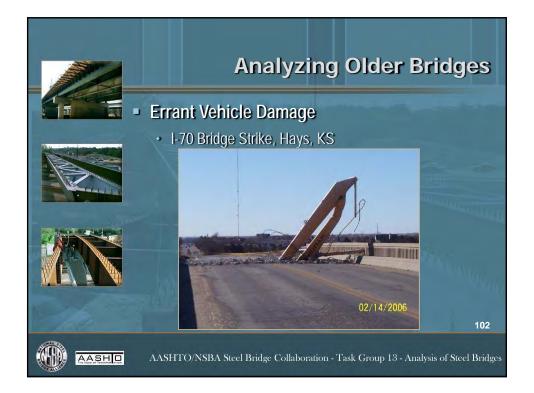


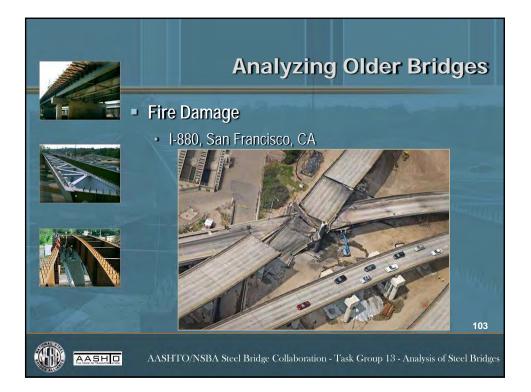


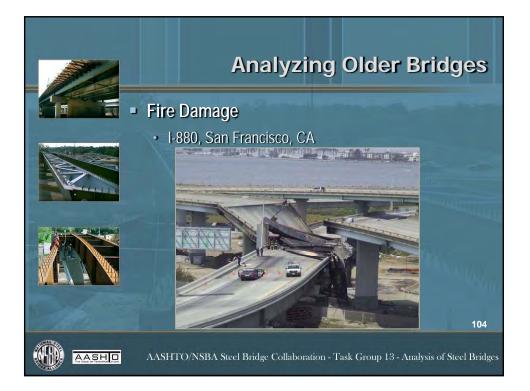


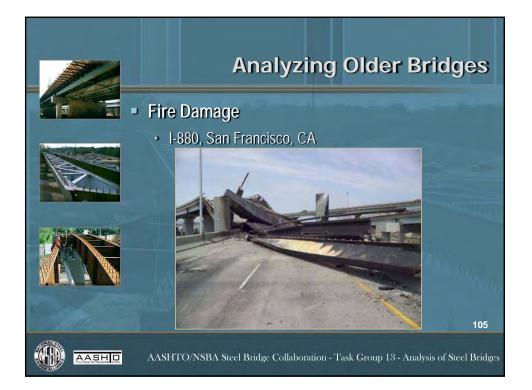


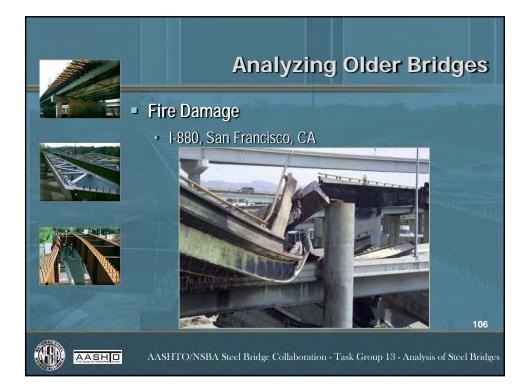




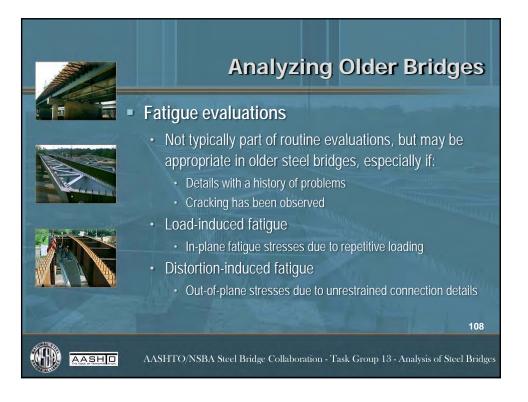


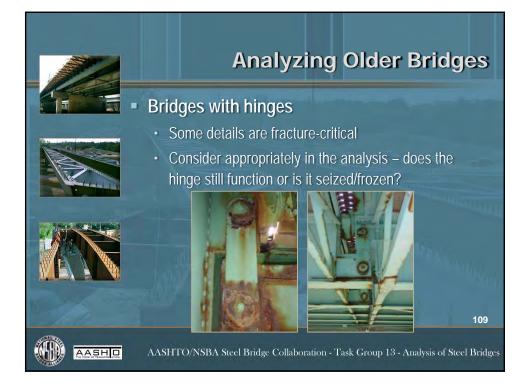




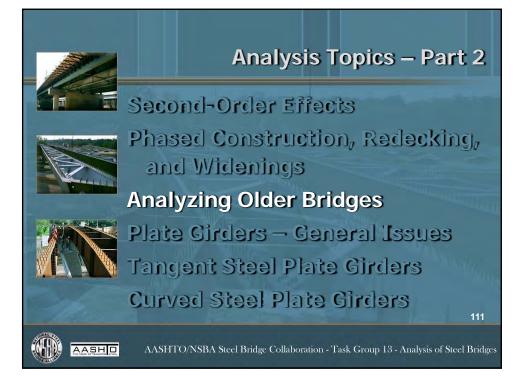


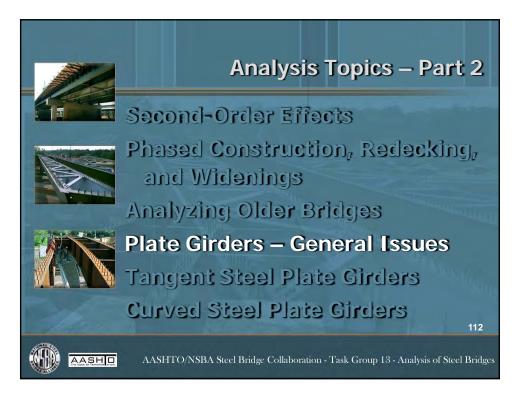


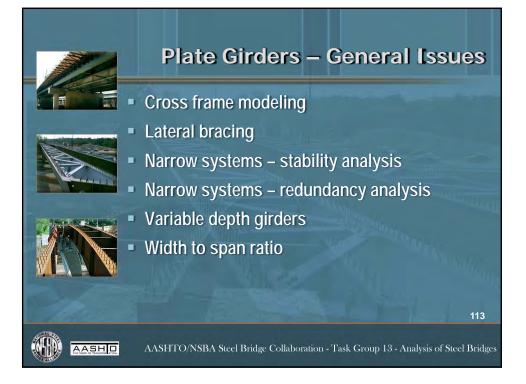


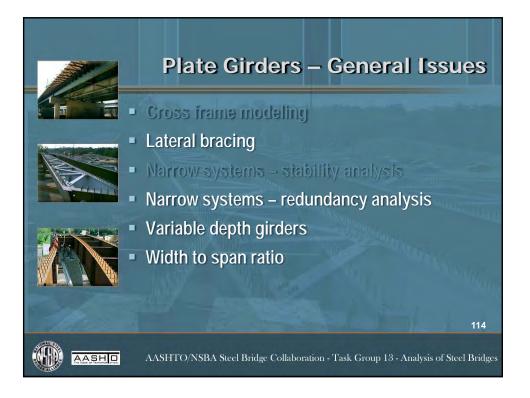


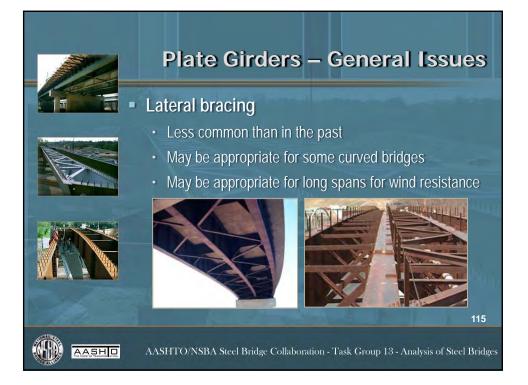
Analyzing Older Bridges
Non-destructive load testing
<ul> <li>Very complex structures</li> <li>Structures with a history of problems</li> </ul>
<ol> <li>Steps</li> <li>Instrument bridge</li> <li>Apply a limited number of known loadings</li> </ol>
<ol> <li>Calibrate analysis model to measured responses</li> <li>Then perform analysis of other load cases</li> </ol>
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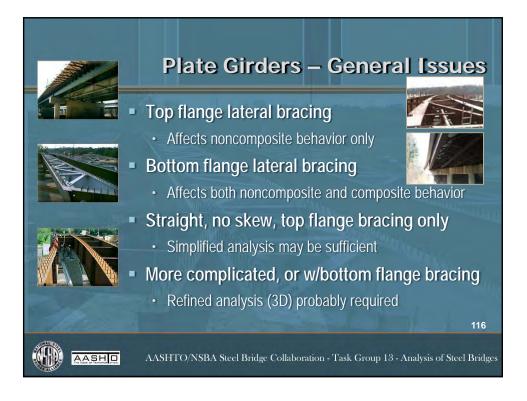












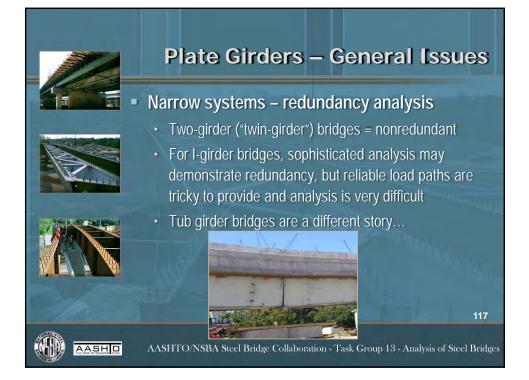
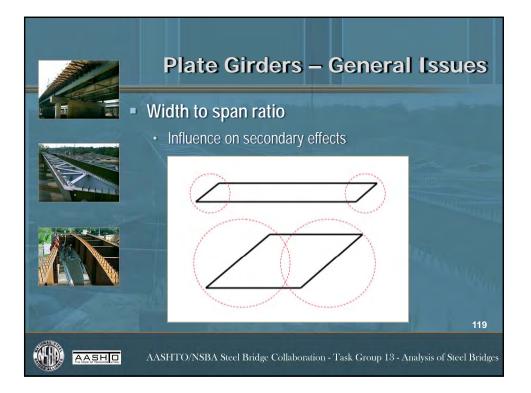
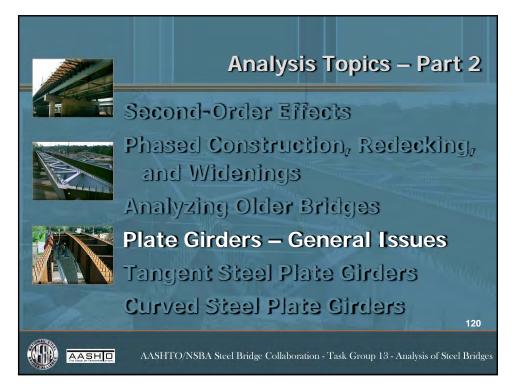


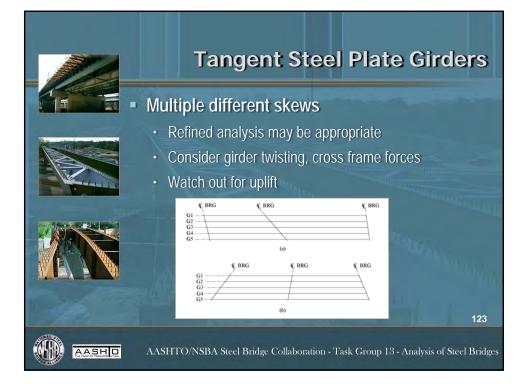
	Plate Girders – General Issues
	Variable depth girders
	Analysis model
	<ul> <li>Stiffness must be modeled correctly or else moment, shear, and dead load deflection predictions will be erroneous</li> </ul>
	Detailed stress analysis
	Refined (3D) analysis
2 h	<ul> <li>Hand analysis per AASHTO LRFD and Blodgett         <sup>118</sup></li> </ul>
	AASHTO/NSBA Steel Bridge Collaboration - Task Group 13 - Analysis of Steel Bridges



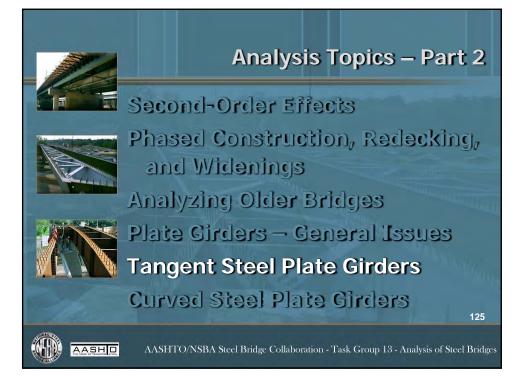


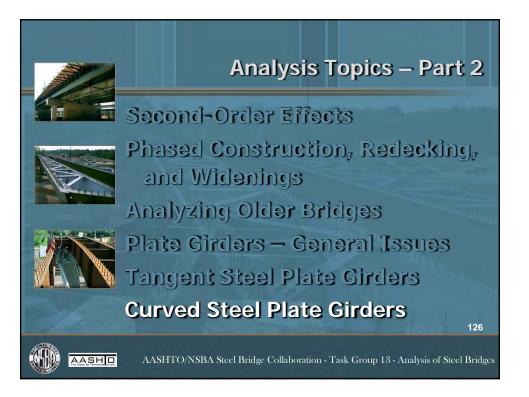




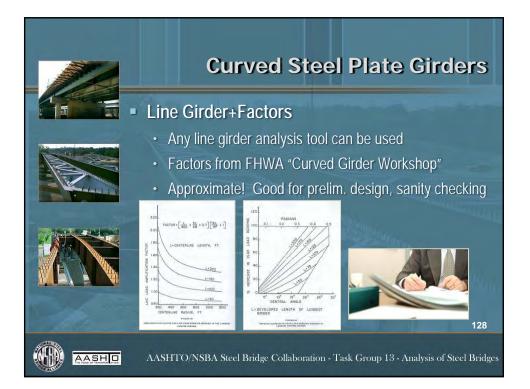


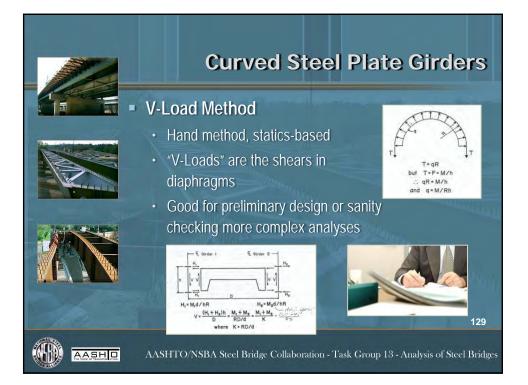


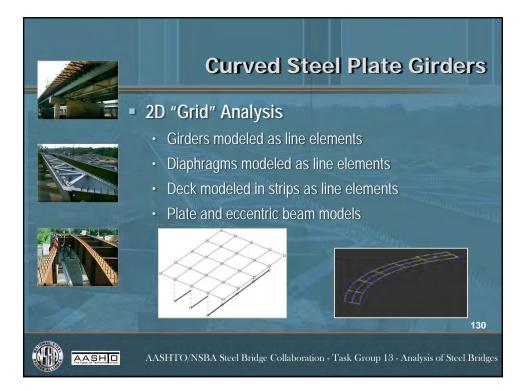


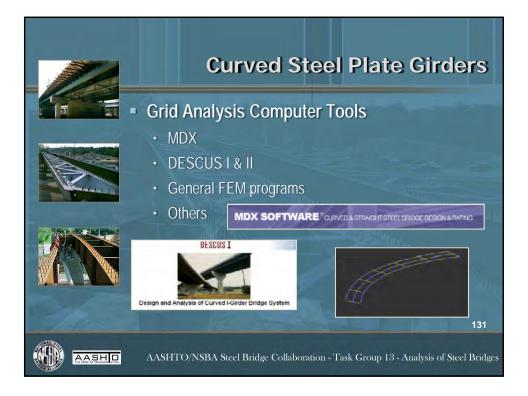


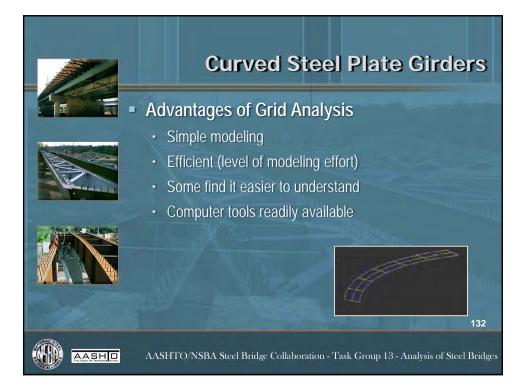


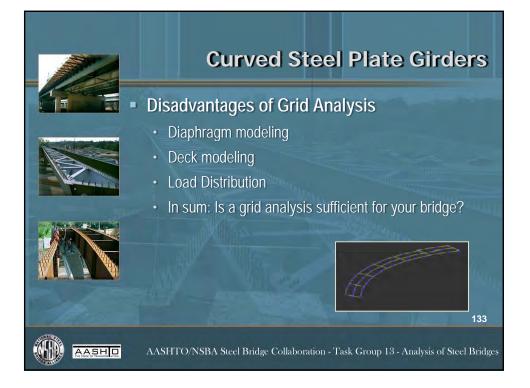


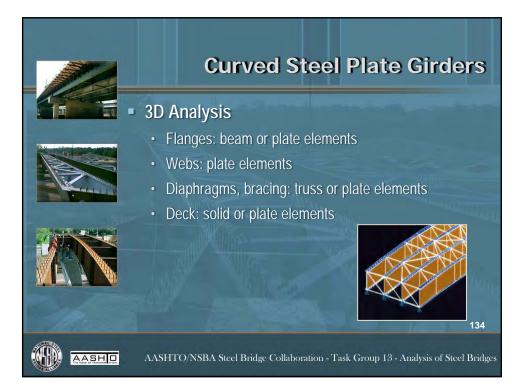




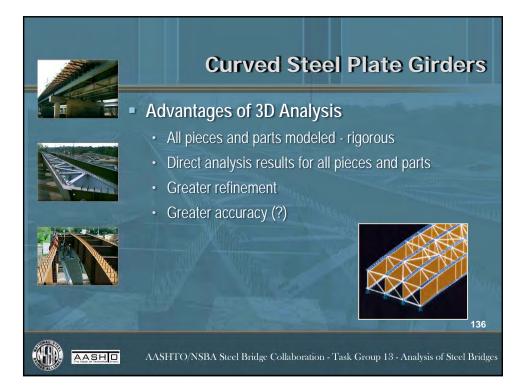


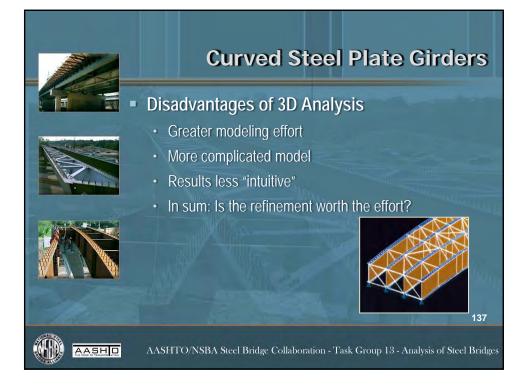












Curved Steel Plate Girders
<ul> <li>Modeling considerations</li> </ul>
<ul> <li>Boundary conditions – modeling bearings</li> <li>Modeling substructure stiffness</li> <li>Modeling connectivity between deck and girders</li> <li>Modeling offsets <ul> <li>Girders to deck</li> <li>Cross frame members to flanges</li> </ul> </li> </ul>
Modeling live loads     138     AASHTO/NSBA Steel Bridge Collaboration - Task Group 13 - Analysis of Steel Bridges

